



Faszination Mauerwerk



SIA 266 Mauerwerk



Kursprogramm

- **21.09.2004**

Statik der Tragsysteme
Prof. Beat Noser

18.40 – 19.00 Pause

**Grundlagen des
Tragverhaltens**
Dr. Joseph Schwarz

- **21.09.2004**

Normalkraftbeanspruchung
Prof. Beat Noser

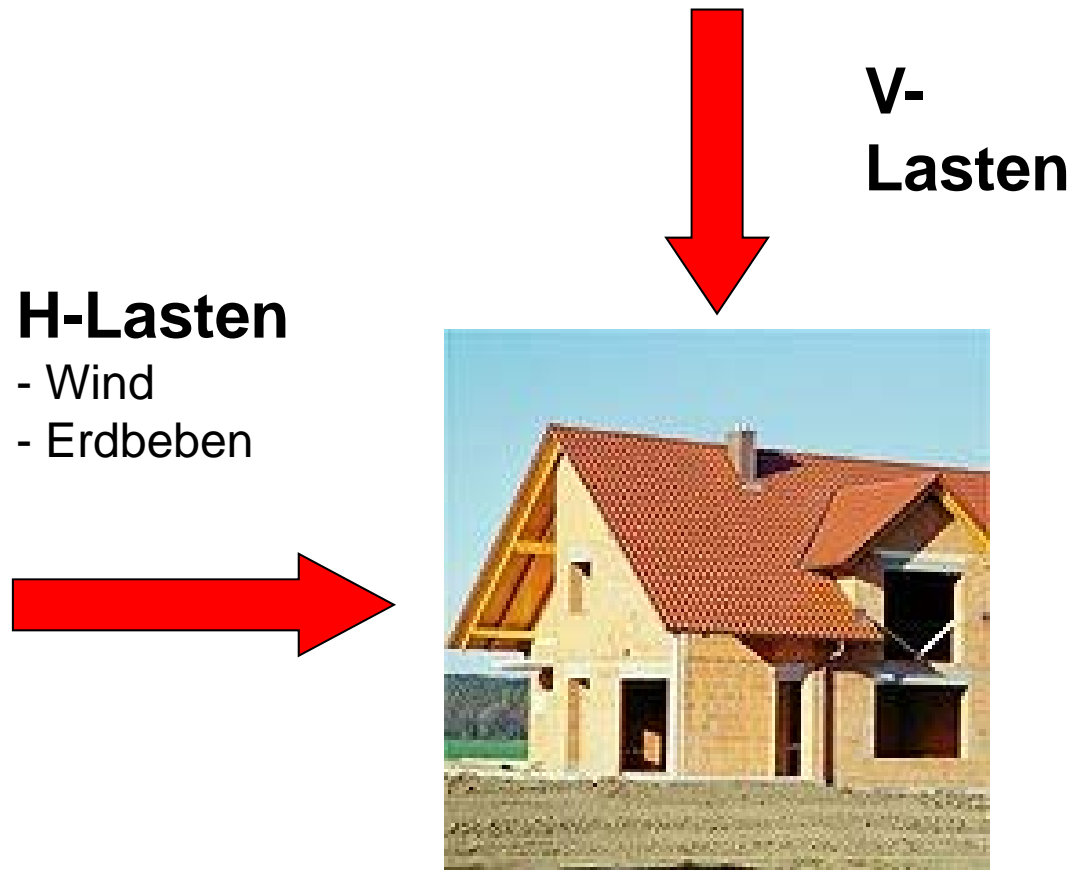
18.40 – 19.00 Pause

Schubbeanspruchung
Dr. Joseph Schwarz

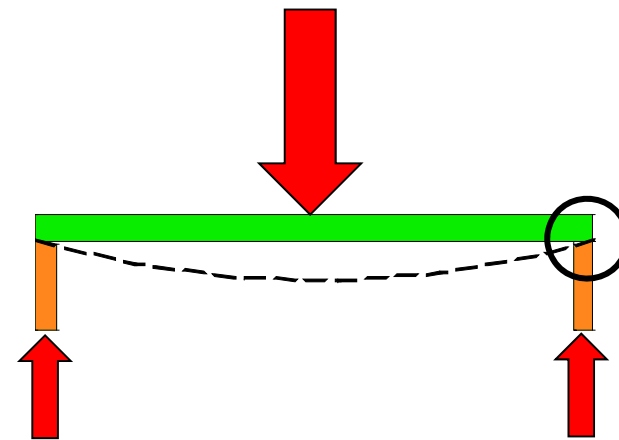
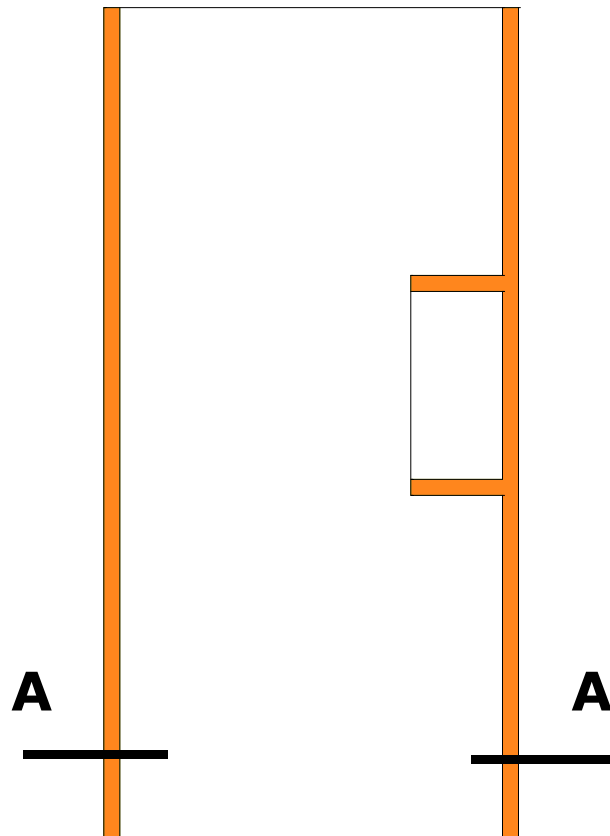
**Armiertes Mauerwerk /
Erdbeben**
Dr. Joseph Schwarz



Statik der Tragsysteme



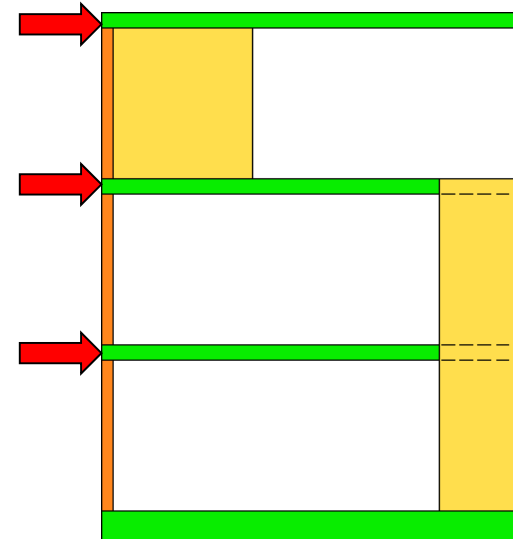
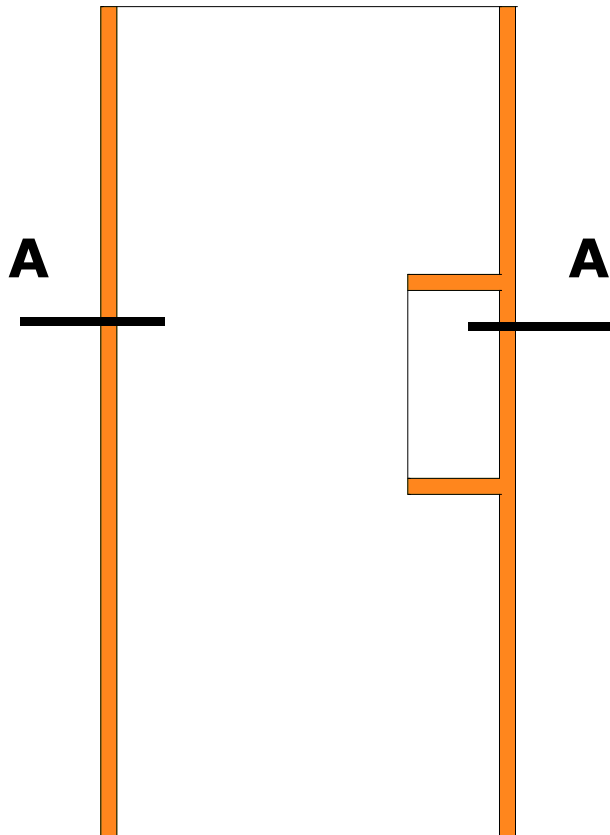
Statik der Tragsysteme V-Lasten



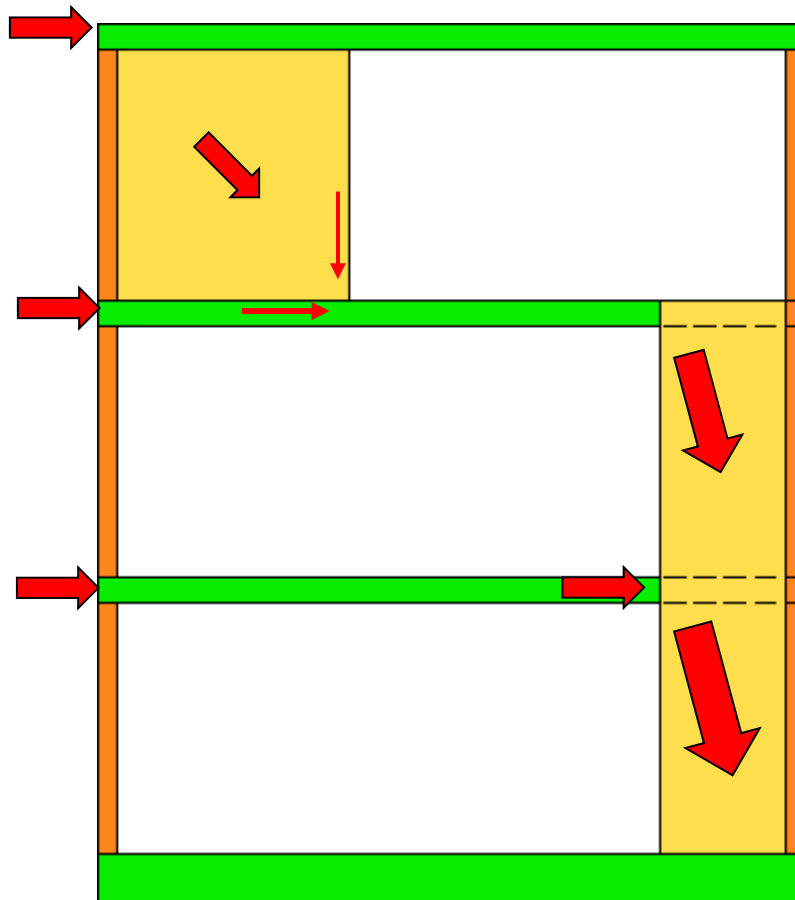
Nachweise:

- Traglast Decke /
Deckendurchbiegung
- Traglast Wand / Wandverdrehung

Statik der Tragsysteme H-Lasten



Statik der Tragsysteme H-Lasten



Lastpfad:

- Schubkräfte im Mauerwerk

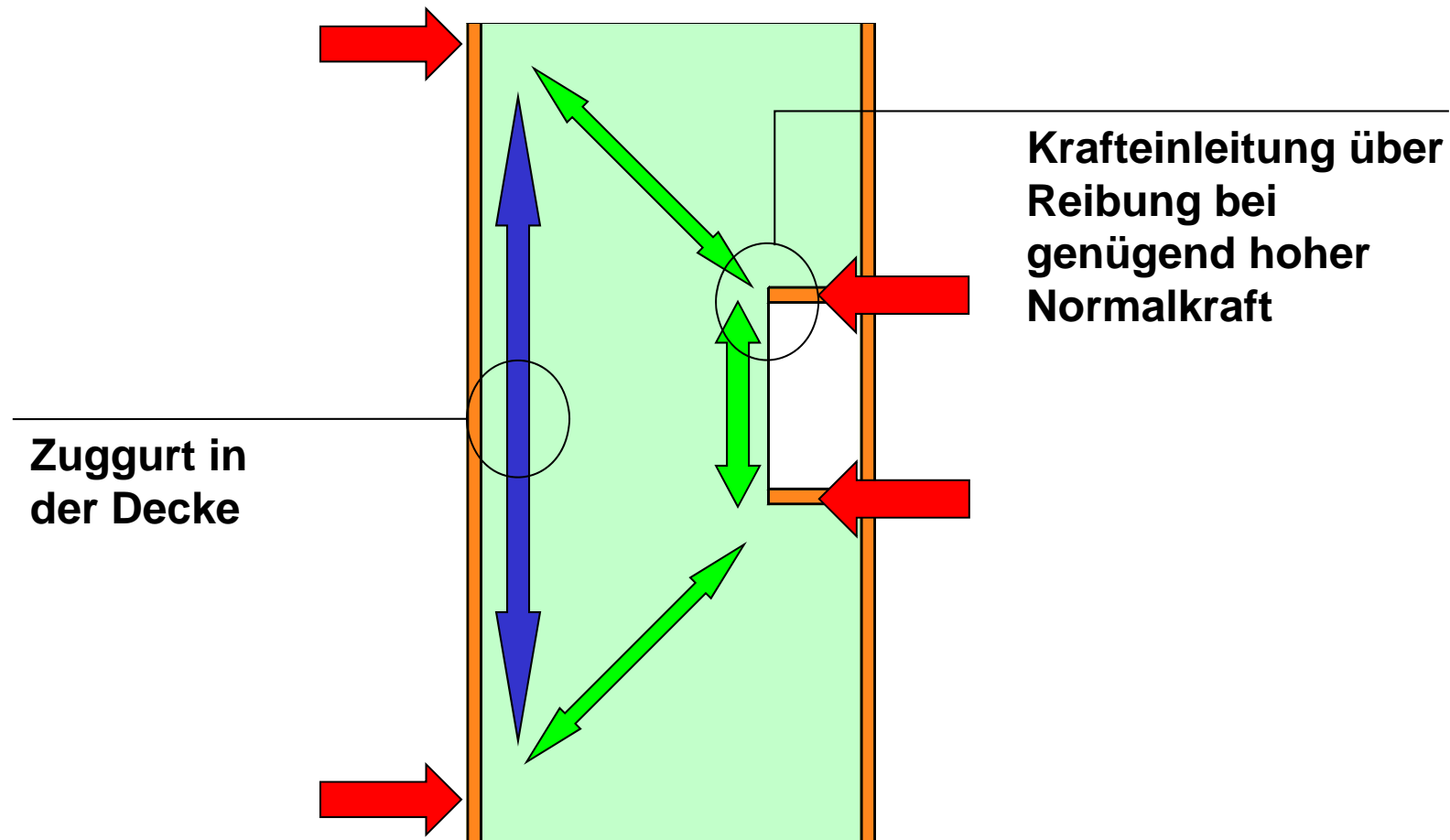
- Horizontalkräfte in der Decke

Randbedingungen:

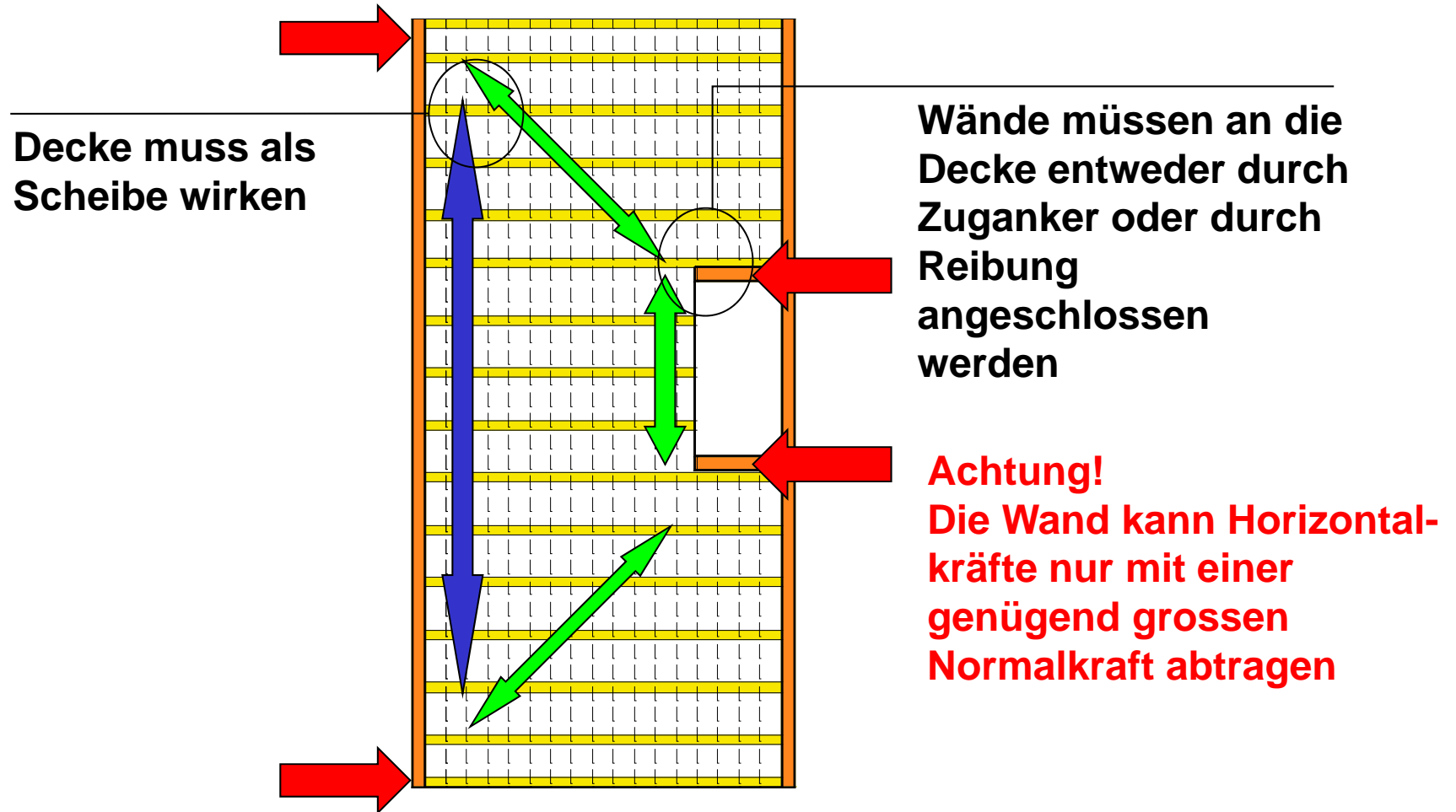
- Mauerwerk überträgt keine Zugspannungen

- Schubsteife Decke

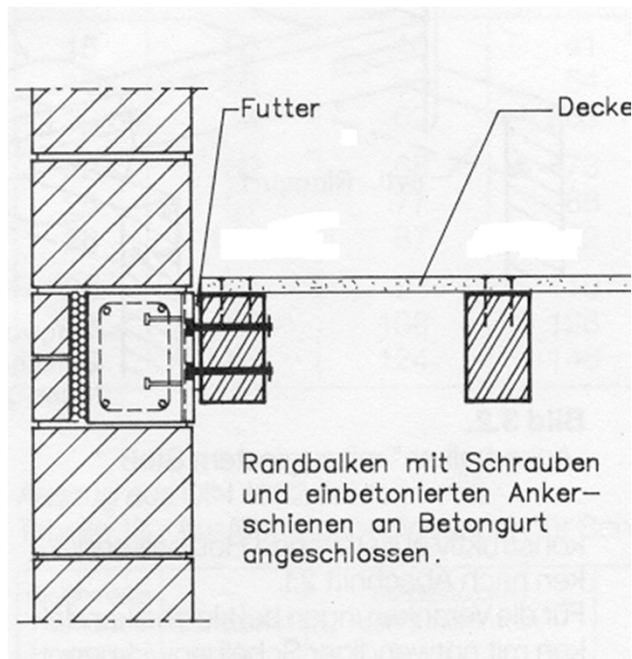
Lastpfad Decke - Wand



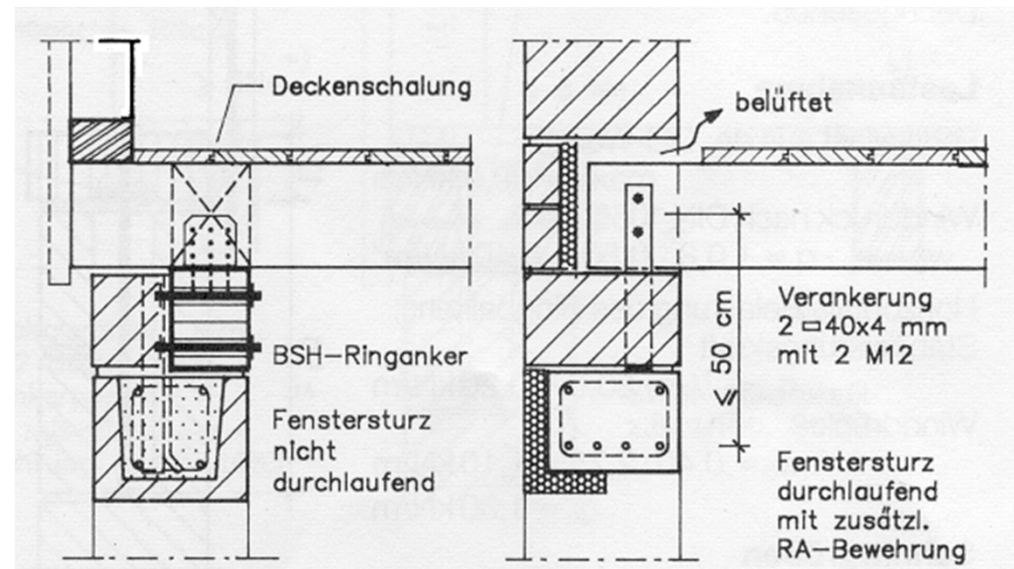
Aussteifende Holzbalkendecken



Ringbalken

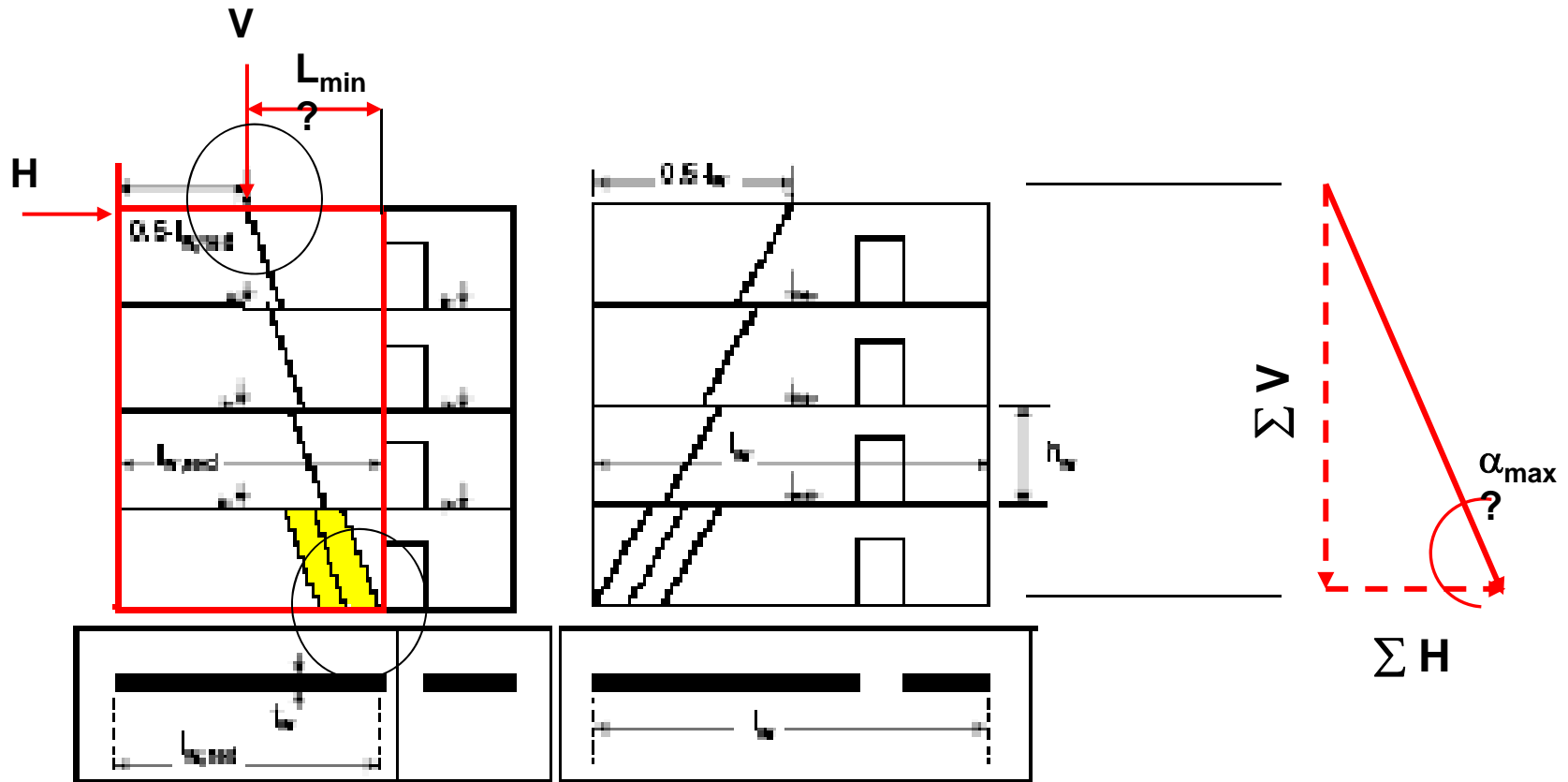


Anschluss parallel zur Wand

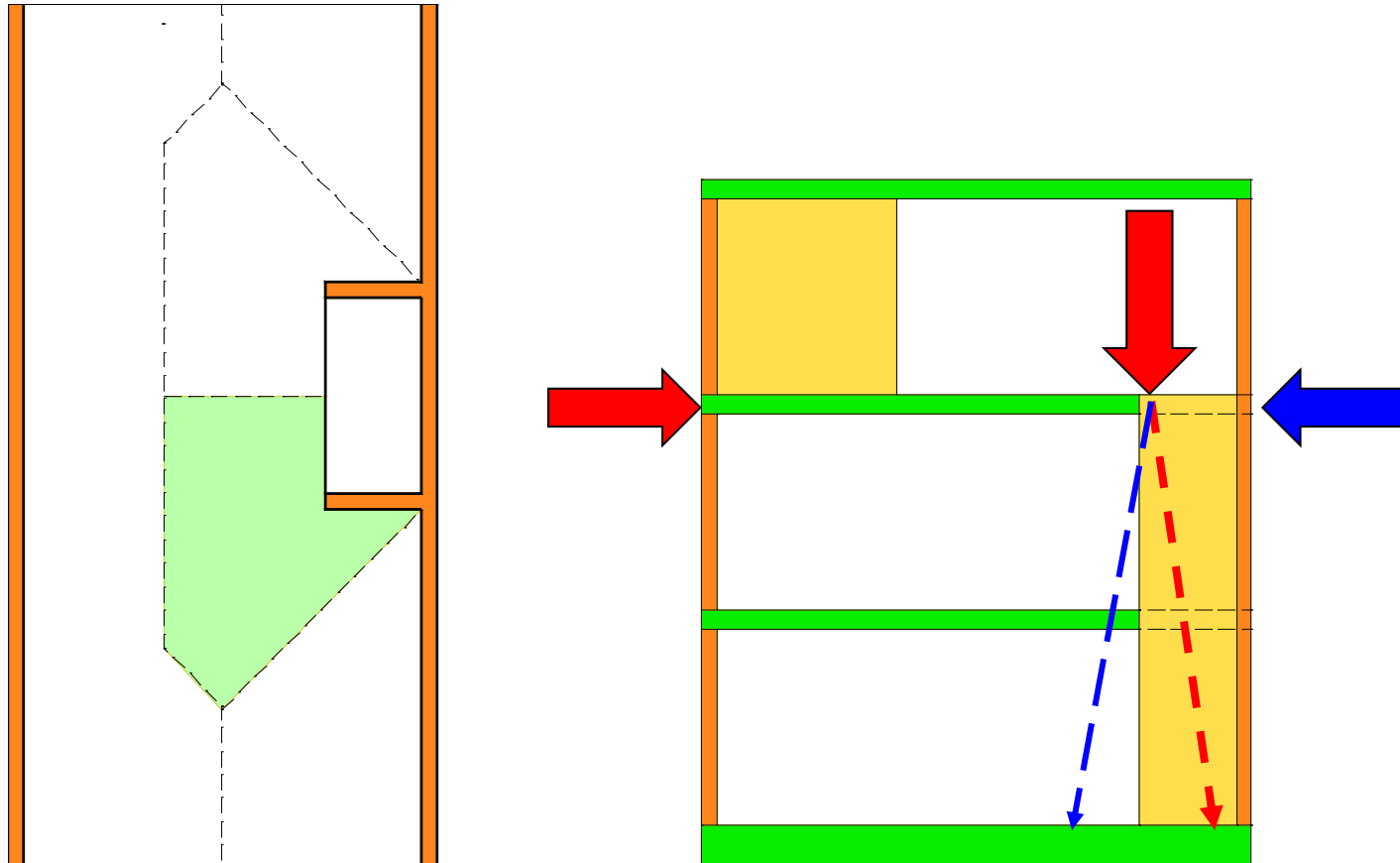


Anschluss senkrecht zur Wand

Lastpfad Wand



Lastpfad Wand



Verteilung der Vertikallasten

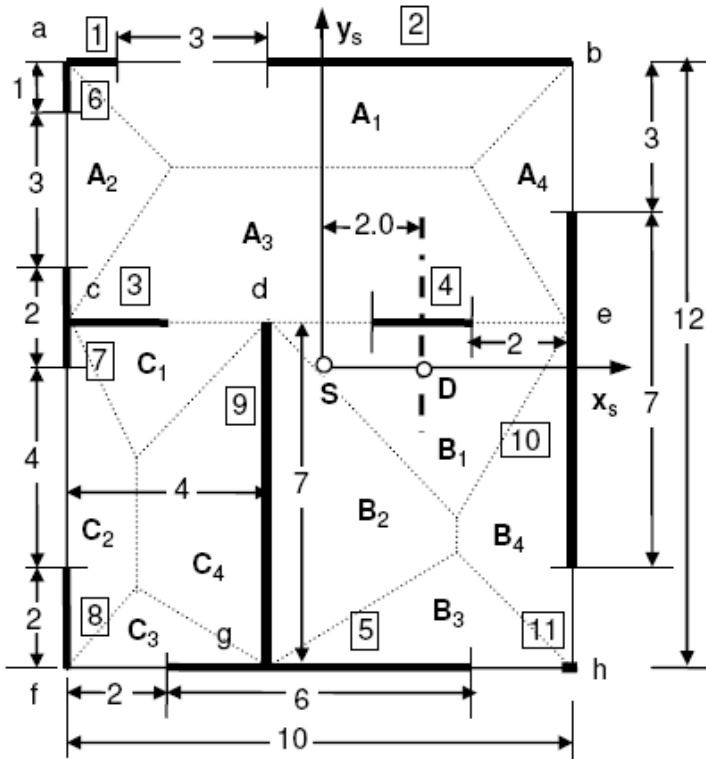


Tabelle 2.2-3: Spezifische Lastezugsflächen der Wandquerschnitte

WQS	Art	Abschnitte		a [m ² /m]	l [m]
1	E	a-b	$1.496 \cdot (1+1.5)/1$	3.740	1.0
2	E	a-b	$1.496 \cdot (1.5+6)/6$	1.870	6.0
3	Z	c-d	$3.857 \cdot (2+1)/2$	5.786	2.0
4	Z	d-e,	$4.492 \cdot (2+1+1)/2$	8.984	2.0
5	E	f-g, g-h	$0.732 \cdot (2+1)/6 + 1.098 \cdot (4+1)/6$	1.281	6.0
6	E	a-c	$0.916 \cdot (1+1.5)/1$	2.290	1.0
7	E	a-c, c-f	$0.916 \cdot (1+1.5)/2 + 1.046 \cdot (1+2)/2$	2.714	2.0
8	E	c-f, f-g	$1.046 \cdot (2+2)/2 + 0.732 \cdot 1/2$	2.458	2.0
9	Z	d-g, c-d, d-e	$3.985 + 3.858 \cdot 1/7 + 4.492 \cdot 1/7$	5.178	7.0
10	E	b-e, e-h, d-e	$0.916 \cdot (2+1.5)/7 + 1.254 \cdot (5+1)/7 + 4.492 \cdot 1/7$	2.175	7.0
11	S	e-h, g-h	$1.098 \cdot 1 + 1.254 \cdot 1$	(2.352)	+ (...)
Summenkontrolle:				a · l	119

Verteilung der Vertikallasten

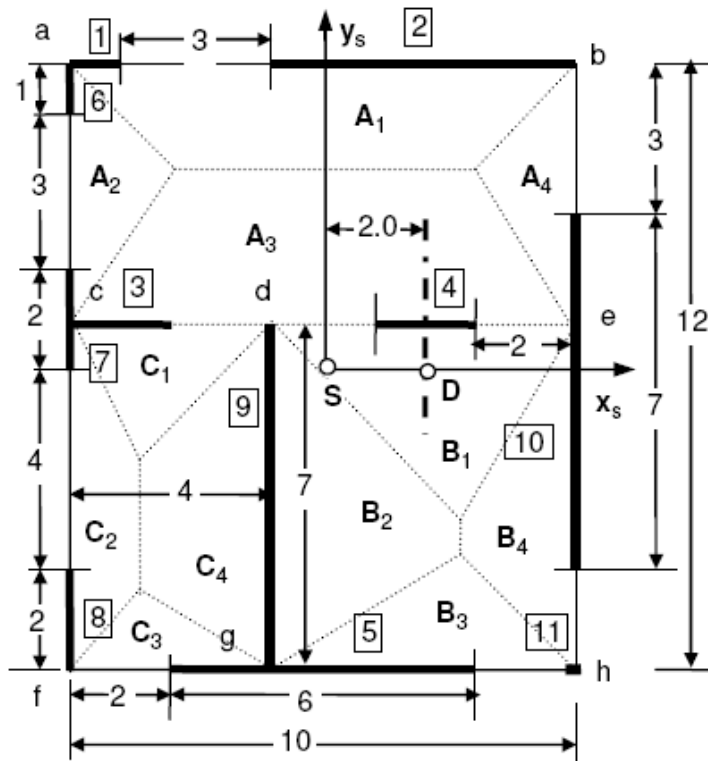
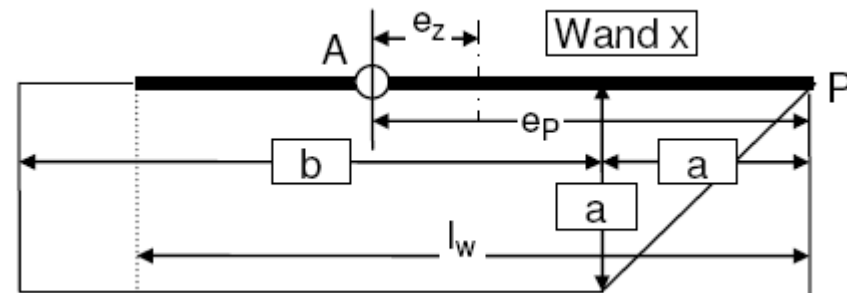
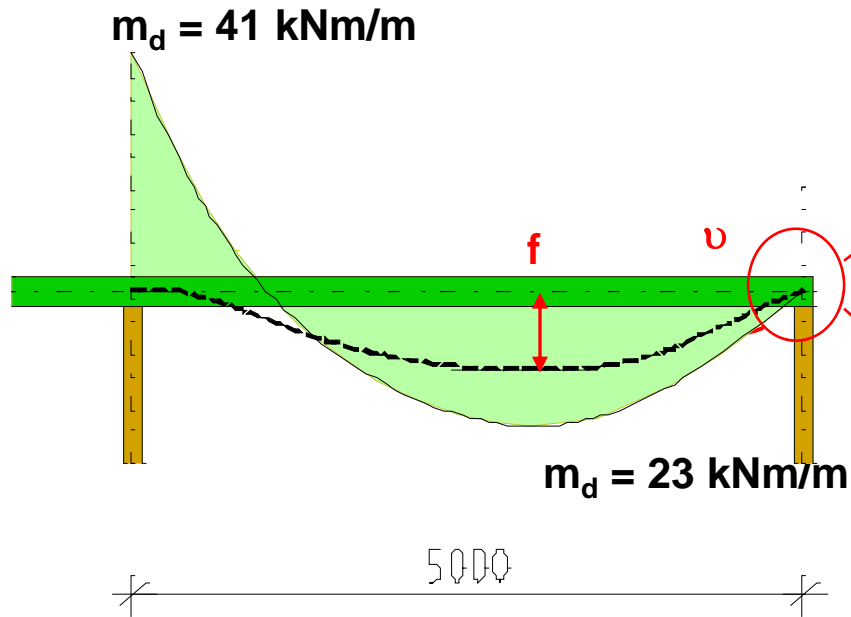


Tabelle 2.2-4: Wandnormalkräfte ΔN_k der Decken alleine in kN/m

WQS	1 E	2 E	3 Z	4 Z	5 E	6 E	7 E	8 E	9 Z	10 E
$\Delta N_{xk,BG}$	28.1	14.0	43.4	67.4	9.6	16.8	20.2	18.4	38.8	16.3
$\Delta N_{xk,PG}$	19.4	18.2	43.9	82.0	11.3	20.4	12.3	18.7	37.2	15.7
$\Delta N_{xk,PN}$	5.2	4.8	11.7	21.9	3.0	5.4	3.3	5.0	9.9	4.2



Deckenverdrehung

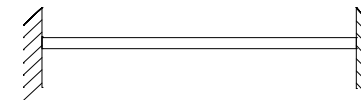


$Q_d = 13.125 \text{ kN/m}^2$
 $M'_d = 41.0 \text{ kNm/m}$
 $M_d = 23.0 \text{ kNm/m}$

} Balkenstatik !



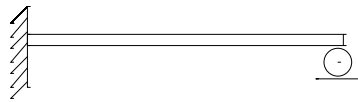
$$v = \frac{q_d \cdot l^4}{48 \cdot EJ} = 5.5 \cdot 10^{-3}$$



$$m = \frac{q_d \cdot l^2}{12} = -27.3 \text{ kNm/m}$$

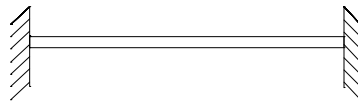
$$v = 0$$

Deckenverdrehung



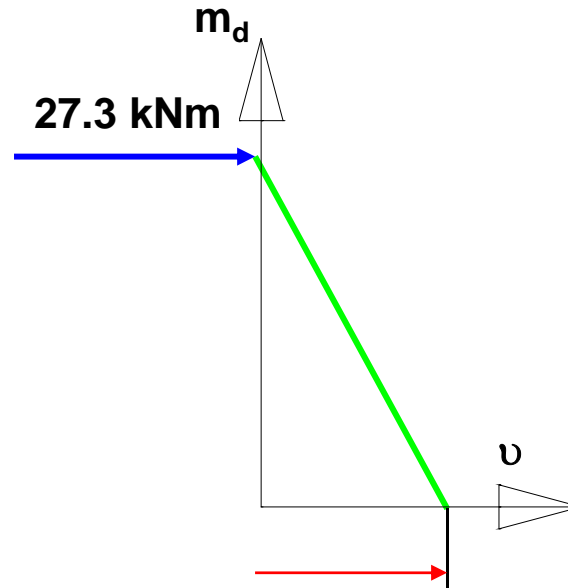
$$m = 0$$

$$v = \frac{q_d \cdot l^4}{48 \cdot EJ} = \underline{5.5 \cdot 10^{-3}}$$



$$m = \frac{q_d \cdot l^2}{12} = \underline{-27.3 \text{ kNm/m}}$$

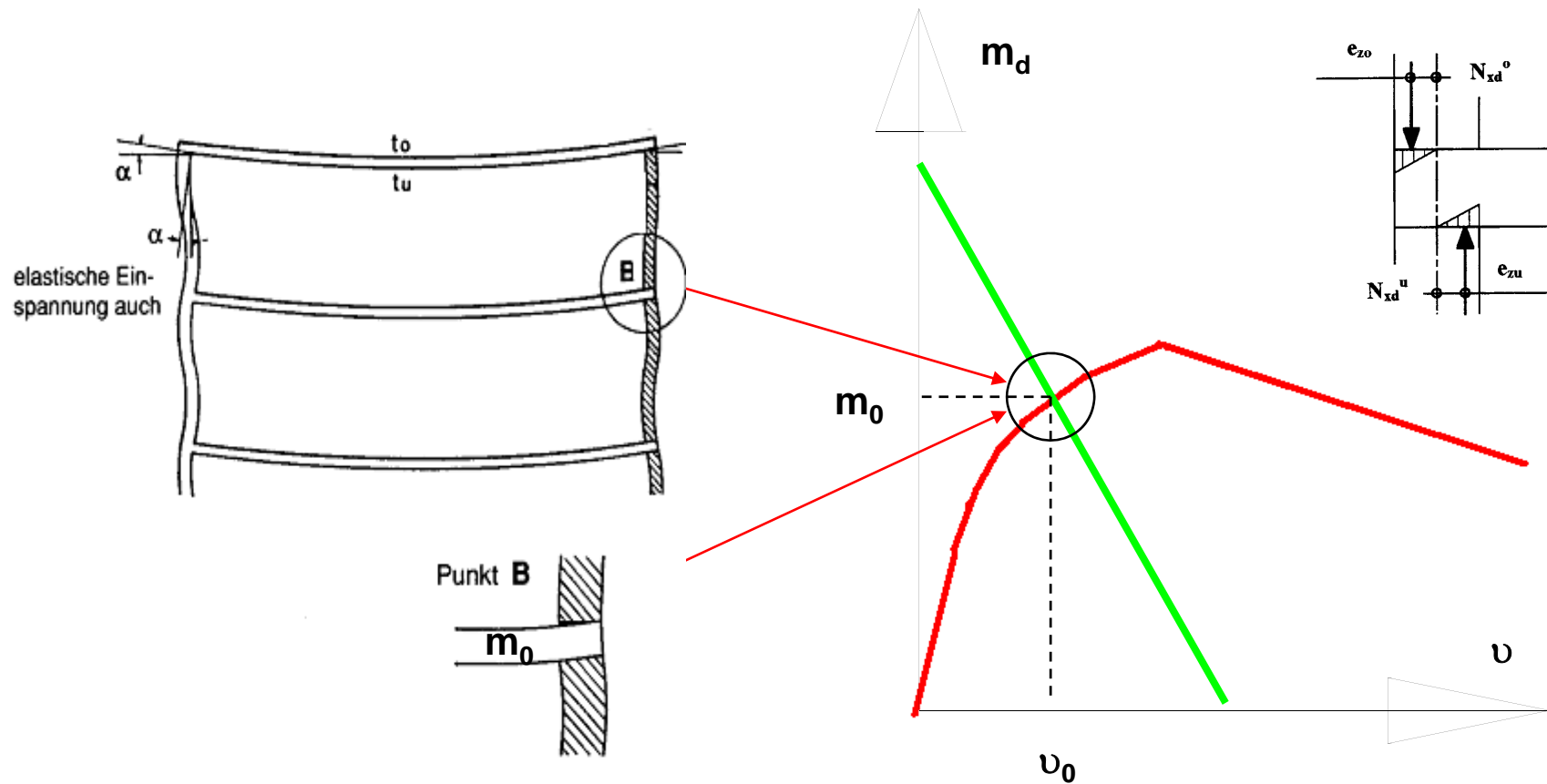
$$v = 0$$



$$v^\infty = v_{el} \cdot \frac{1 - 20\rho'}{10 \cdot \rho^{0.7}} \cdot (0.75 + 0.1\varphi) \cdot \left(\frac{h}{d}\right)^3$$

$$v^\infty = 5.5 \cdot 10^{-3} \frac{1}{10 \cdot 0.0028^{0.7}} \cdot (0.75 + 0.1) \left(\frac{0.22}{0.17}\right)^3 = 62 \cdot 10^{-3}$$

Verträglichkeit Wand - Decke



Verträglichkeit Wand - Decke

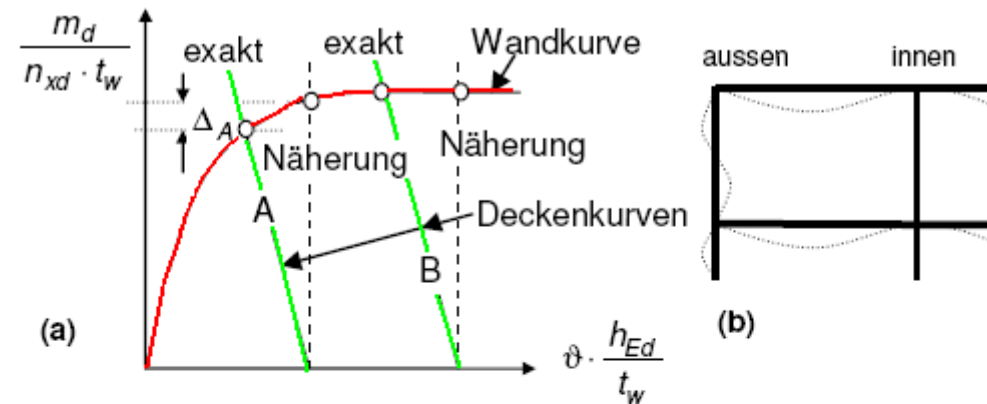


Bild 2.3-2 Deckenkurve und Wandkurve

Bildbezeichnungen

m_d Decken- bzw. Wandmoment pro Laufmeter

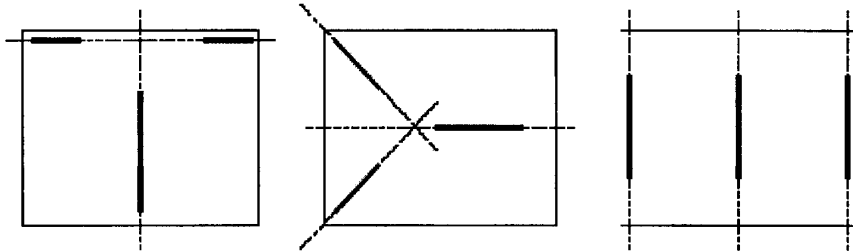
n_{xd} Wandnormalkraft pro Laufmeter

t_w Wandstärke

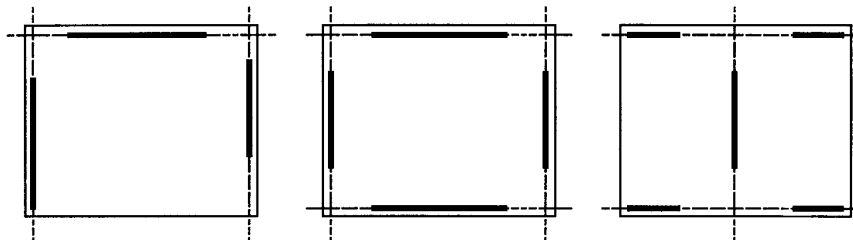
ϑ Verdrehung im Knoten Wand – Decke

h_{Ed} Bezugshöhe der Wand

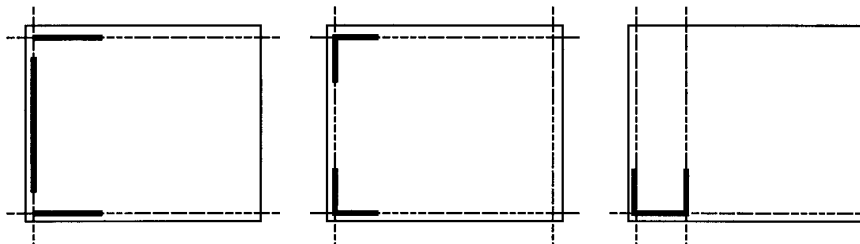
Verteilung der Horizontallasten



Instabile Anordnung der aussteifenden Wände

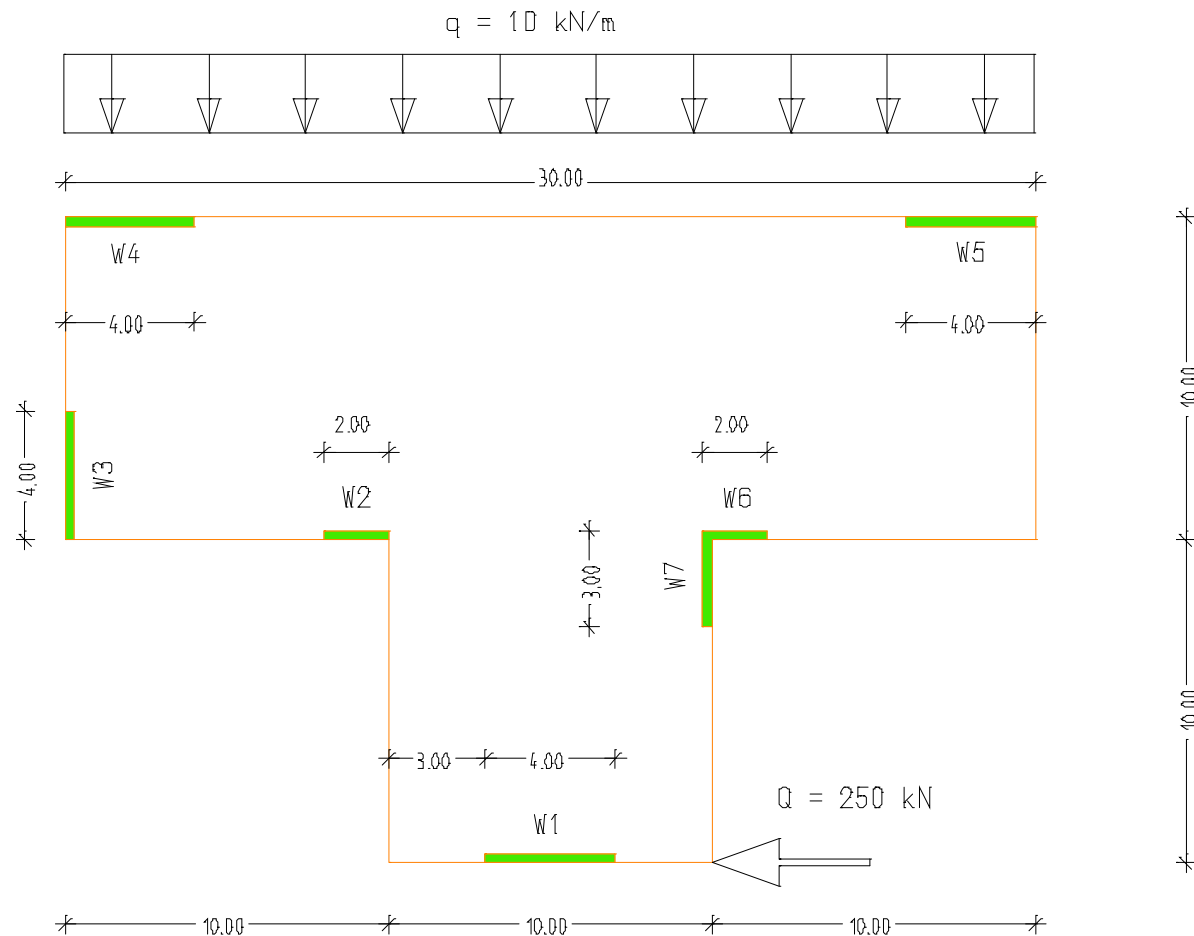


Stabile Anordnung der aussteifenden Wände



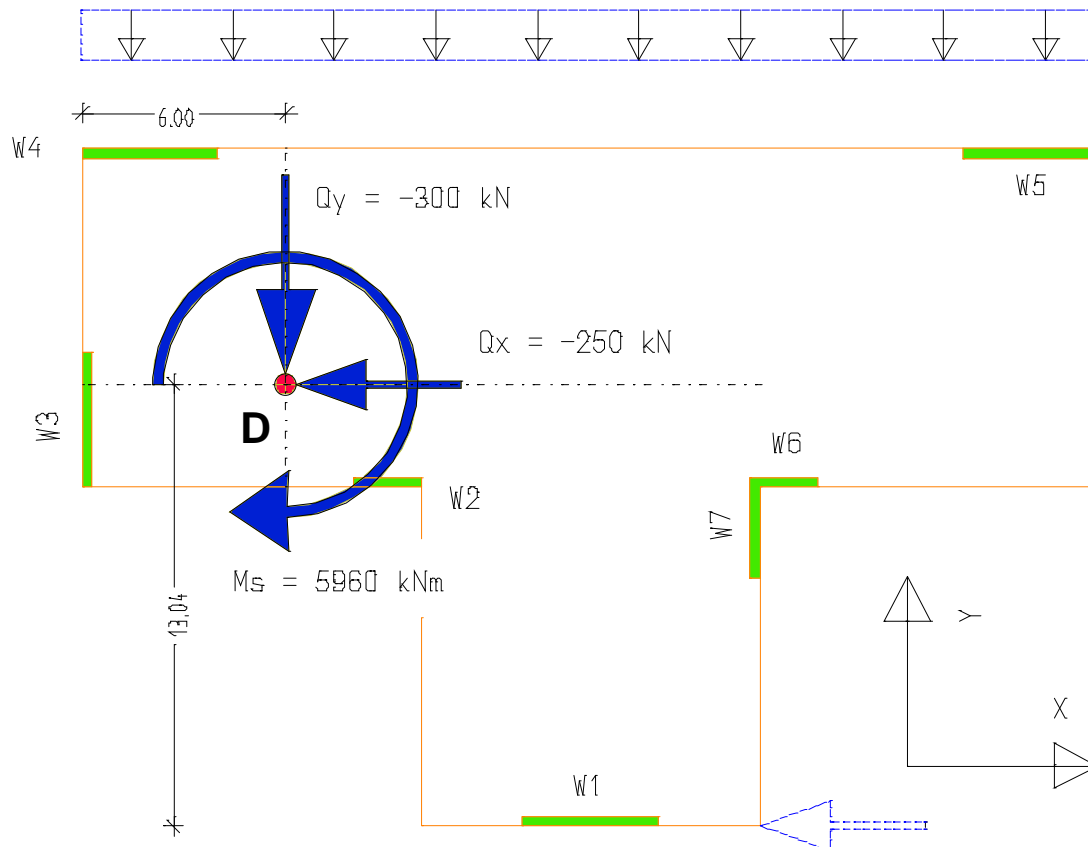
Stabile, aber ungünstige Anordnung der aussteifenden Wände

Verteilung der Horizontallasten Schubmittelpunkt



Verteilung der Horizontallasten Schubmittelpunkt

$q = 10 \text{ kN/m}$

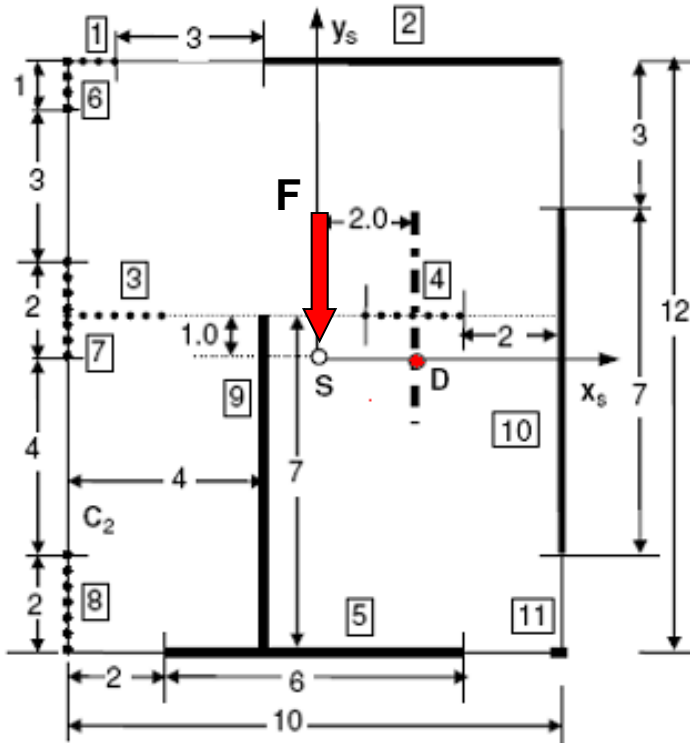


$$x_D = \frac{\sum J_{xi} \cdot x_i}{\sum J_{xi}}$$

$$y_D = \frac{\sum J_{yi} \cdot y_i}{\sum J_{yi}}$$



Verteilung der Horizontallasten Schubmittelpunkt



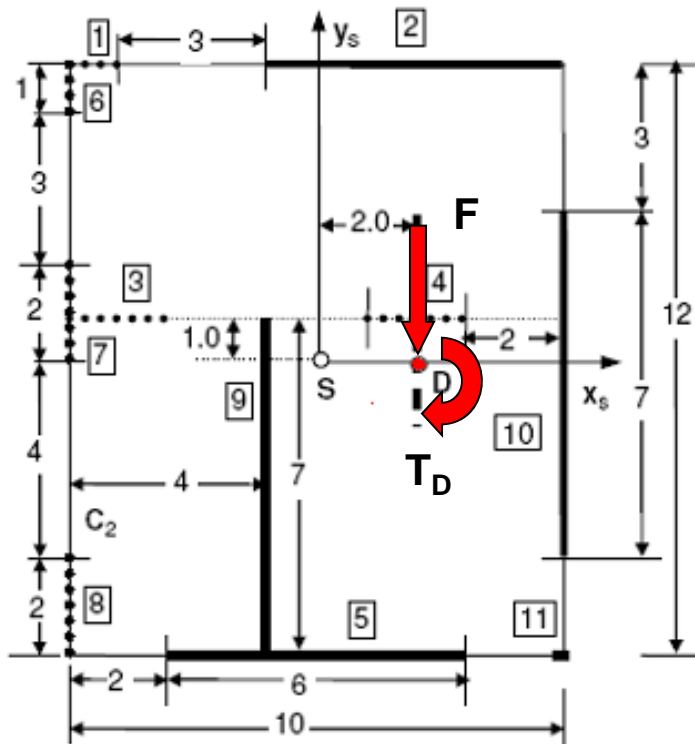
Wand 2
 $x_2 = 7.0$; $y_2 = 12.0$; $L = 6.0$

Wand 5
 $x_5 = 5.0$; $y_2 = 0.0$; $L = 6.0$

Wand 9
 $x_9 = 4.0$; $y_9 = 3.5$; $L = 7.0$

Wand 10
 $x_{10} = 10.0$; $y_{10} = 5.5$; $L = 7.0$

Verteilung der Horizontallasten Schubmittelpunkt



Einwirkungen im
Schubmittelpunkt:

Kraft = $-F_y$; Moment $M = 2 F$

Auswirkungen:

Inf. F : $W_9 = W_{10} = 0.50 F$

$$\text{Inf. } T_D \quad V_{xi,T} = \frac{-T_D \cdot y_i' \cdot J_{yi}}{\Sigma \cdot (y_i'^2 \cdot J_{yi} + x_i'^2 \cdot J_{xi})}$$

$$V_{yi,T} = \frac{T_D \cdot x_i' \cdot J_{xi}}{\Sigma \cdot (y_i'^2 \cdot J_{yi} + x_i'^2 \cdot J_{xi})}$$